

TABLE 12.6 EQUIVALENT STRESSES FOR SLABS WITHOUT CONCRETE SHOULDERS

Slab thickness (in.)	<i>k</i> of Subgrade-subbase (pci)						
	50	100	150	200	300	500	700
4	825/679	726/585	671/542	634/516	584/486	523/457	484/443
4.5	699/586	616/500	571/460	540/435	498/406	448/378	417/363
5	602/516	531/436	493/399	467/376	432/349	390/321	363/307
5.5	526/461	464/387	431/353	409/331	379/305	343/278	320/264
6	465/416	411/348	382/316	362/296	336/271	304/246	285/232
6.5	417/380	367/317	341/286	324/267	300/244	273/220	256/207
7	375/349	331/290	307/262	292/244	271/222	246/199	231/186
7.5	340/323	300/268	279/241	265/224	246/203	224/181	210/169
8	311/300	274/249	255/223	242/208	225/188	205/167	192/155
8.5	285/281	252/232	234/208	222/193	206/174	188/154	177/143
9	264/264	232/218	216/195	205/181	190/163	174/144	163/133
9.5	245/248	215/205	200/183	190/170	176/153	161/134	151/124
10	228/235	200/193	186/173	177/160	164/144	150/126	141/117
10.5	213/222	187/183	174/164	165/151	153/136	140/119	132/110
11	200/211	175/174	163/155	154/143	144/129	131/113	123/104
11.5	188/201	165/165	153/148	145/136	135/122	123/107	116/98
12	177/192	155/158	144/141	137/130	127/116	116/102	109/93
12.5	168/183	147/151	136/135	129/124	120/111	109/97	103/89
13	159/176	139/144	129/129	122/119	113/106	103/93	97/85
13.5	152/168	132/138	122/123	116/114	107/102	98/89	92/81
14	144/162	125/133	116/118	110/109	102/98	93/85	88/78

Note. Number at left is for single axle and number at right is for tandem axle (single/tandem); 1 in. = 25.4 mm, 1 pci = 271.3 kN/m³.

Source. After PCA (1984).

Example 12.2

The sensitivity analysis presented in Table 5.13 shows an average edge stress of about 283 psi (1.95 MPa) under a 36-kip (160-kN) tandem-axle load when $h = 8$ in. (203 mm) and $k = 100$ pci (27.1 MN/m³). Assuming a concrete modulus of rupture of 500 psi (4.5 MPa) and using the PCA fatigue equation, or Eq. 5.36, determine the allowable number of load repetitions. Compare the result with that obtained from Table 12.6 and Figure 12.12.

Solution: Because only a small portion of wheel loads is applied at the pavement edge, the edge stress must be multiplied by an adjustment factor of 0.894, so the actual stress is $\sigma = 0.894 \times 283 = 253$ psi (1.75 MPa). With $\sigma/S_c = 253/500 = 0.506$, from Eq. 5.36b, $N_f = [4.2577/(0.506 - 0.43250)]^{3.268} = 5.8 \times 10^5$.

From Table 12.6, the equivalent stress for a tandem-axle load with a k value of 100 pci (27.1 MN/m³) and a slab thickness of 8 in. (203 mm) is 249 psi (1.72 MPa), which is slightly smaller than the 253 psi (1.75 MPa) obtained in the sensitivity analysis. This is as expected because the dual and tandem spacings as well as the tire contact pressure assumed by PCA is not known and may be quite

TABLE 12.11 EROSION FACTORS FOR SLABS WITH AGGREGATE INTERLOCK JOINTS AND CONCRETE SHOULDERS

Slab thickness (in.)	<i>k</i> of Subgrade-subbase (pci)					
	50	100	200	300	500	700
4	3.46/3.49	3.42/3.39	3.38/3.32	3.36/3.29	3.32/3.26	3.28/3.24
4.5	3.32/3.39	3.28/3.28	3.24/3.19	3.22/3.16	3.19/3.12	3.15/3.09
5	3.20/3.30	3.16/3.18	3.12/3.09	3.10/3.05	3.07/3.00	3.04/2.97
5.5	3.10/3.22	3.05/3.10	3.01/3.00	2.99/2.95	2.96/2.90	2.93/2.86
6	3.00/3.15	2.95/3.02	2.90/2.92	2.88/2.87	2.86/2.81	2.83/2.77
6.5	2.91/3.08	2.86/2.96	2.81/2.85	2.79/2.79	2.76/2.73	2.74/2.68
7	2.83/3.02	2.77/2.90	2.73/2.78	2.70/2.72	2.68/2.66	2.65/2.61
7.5	2.76/2.97	2.70/2.84	2.65/2.72	2.62/2.66	2.60/2.59	2.57/2.54
8	2.69/2.92	2.63/2.79	2.57/2.67	2.55/2.61	2.52/2.53	2.50/2.48
8.5	2.63/2.88	2.56/2.74	2.51/2.62	2.48/2.55	2.45/2.48	2.43/2.43
9	2.57/2.83	2.50/2.70	2.44/2.57	2.42/2.51	2.39/2.43	2.36/2.38
9.5	2.51/2.79	2.44/2.65	2.38/2.53	2.36/2.46	2.33/2.38	2.30/2.33
10	2.46/2.75	2.39/2.61	2.33/2.49	2.30/2.42	2.27/2.34	2.24/2.28
10.5	2.41/2.72	2.33/2.58	2.27/2.45	2.24/2.38	2.21/2.30	2.19/2.24
11	2.36/2.68	2.28/2.54	2.22/2.41	2.19/2.34	2.16/2.26	2.14/2.20
11.5	2.32/2.65	2.24/2.51	2.17/2.38	2.14/2.31	2.11/2.22	2.09/2.16
12	2.28/2.62	2.19/2.48	2.13/2.34	2.10/2.27	2.06/2.19	2.04/2.13
12.5	2.24/2.59	2.15/2.45	2.09/2.31	2.05/2.24	2.02/2.15	1.99/2.10
13	2.20/2.56	2.11/2.42	2.04/2.28	2.01/2.21	1.98/2.12	1.95/2.06
13.5	2.16/2.53	2.08/2.39	2.00/2.25	1.97/2.18	1.93/2.09	1.91/2.03
14	2.13/2.51	2.04/2.36	1.97/2.23	1.93/2.15	1.89/2.06	1.87/2.00

Note. Number at left is for single axle and number at right is for tandem axle (single/tandem); 1 in. = 25.4 mm, 1 pci = 271.3 kN/m³.

Source. After PCA (1984).

Explanation of Worksheet

1. Single-axle loads are incremented at 2-kip (8.9-kN) intervals, and tandem-axle loads are incremented at 4-kip (17.8-kN) intervals. The largest load in the single- or tandem-load group should be entered first. If the allowable number of repetitions for a given load is unlimited, it is not necessary to compute the damage for the remaining loads in the same group.
2. The axle loads in column 1 are multiplied by a load safety factor of 1.2.
3. The predicted or expected repetitions are obtained from Table 12.5. To be on the conservative side, the upper limit of the load in the range is used to represent the range. For example, all axle loads between 28 and 30 kip (125 and 134 kN) are considered as 30 kip (134 kN). With an annual growth rate of 4% and a design period of 20 years, from Table 6.12, growth factor $G = 1.5$. Design ADT = $12,900 \times 1.5 = 19,350$, or 9675 in one direction. ADTT = $19,350 \times 0.19 = 3680$, or 1840 in one direction. For an ADT of 9675 in one direction, from Figure 6.8, lane distribution factor $L = 0.81$. Therefore, the total number of trucks on the design lane during the design period is $1840 \times$

Calculation of Pavement Thickness

Project Design 1A, four-lane Interstate, rural
 Trial thickness 9.5 in. Doweled joints: yes no
 Subbase-subgrade k 130 pci Concrete shoulder: yes no
 Modulus of rupture, MR 650 psi Design period 20 years
 Load safety factor, LSF 1.2 4 in. untreated subbase

Axle load, kips	Multiplied by LSF <u>1.2</u>	Expected repetitions	Fatigue analysis		Erosion analysis	
			Allowable repetitions	Fatigue, percent	Allowable repetitions	Damage percent
1	2	3	4	5	6	7

8. Equivalent stress 206 10. Erosion factor 2.59
 9. Stress ratio factor 0.317

Single Axles

30	36.0	6,310	27,000	23.3	1,500,000	2.2
28	33.6	14,690	77,000	19.1	2,200,000	0.7
26	31.2	30,140	230,000	13.1	3,500,000	0.9
24	28.8	64,410	1,200,000	5.4	5,900,000	1.1
22	26.4	106,900	Unlimited	0	11,000,000	1.0
20	24.0	235,800	"	0	23,000,000	1.0
18	21.6	507,200	"	0	64,000,000	0.8
16	19.2	422,500			Unlimited	0
14	16.8	586,900			"	0
12	14.4	1,837,000			"	0

11. Equivalent stress 192 13. Erosion factor 2.79
 12. Stress ratio factor 0.295

Tandem Axles

52	62.4	21,320	1,100,000	1.9	920,000	2.3	
48	57.6	42,870	Unlimited	0	1,500,000	2.9	
44	52.8	124,900	"	0	2,500,000	5.0	
40	48.0	372,900	"	0	4,600,000	8.1	
36	43.2	885,800			9,500,000	9.3	
32	38.4	930,200			24,000,000	3.9	
28	33.6	1,656,000			92,000,000	1.6	
24	28.8	984,900			Unlimited	0	
20	24.0	1,227,000			"	0	
16	19.2	1,356,000					
Total					<u>62.8</u>	Total <u>38.2</u>	

Figure 12.15 Worksheet for sample problem (1 in. = 25.4 mm, 1 psi = 6.9 kPa, 1 pci = 271.3 kN/m³). (After PCA (1984).)

Pavement Data		Axle Data			
Trial Thickness, h (in)	9.5	Axle Load (kips)		Axle Load (kips)	
Subgrade Modulus, K (psi/in)	130	Single	Axle/1000	Tandem	Axle/1000
Modulus of Rupture, Sc (psi)	650	30	0.58	52	1.96
Concrete Shoulder <input type="radio"/> Yes <input checked="" type="radio"/> No		28	1.35	48	3.94
Doweled Joints <input checked="" type="radio"/> Yes <input type="radio"/> No		26	2.77	44	11.48
OK		24	5.92	40	34.27
Cancel		22	9.83	36	81.42
		20	21.67	32	85.50
		18	28.24	28	152.21
		16	38.83	24	90.52
		14	53.94	20	112.78
		12	168.04	16	124.63
		OK Cancel			

Traffic Data	
Average Daily Truck Traffic, ADTT	3680
Design Period, Years	20
Load Safety Factor, LSF	1.2
OK Cancel	

圖一 PCAWIN 基本資料輸入視窗圖例

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PCA Thickness Design
File Edit Perform Analysis Compute Design Thickness Help

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*****
PCA Thickness Design
Original Data
*****

Trial Thickness, h (in) = 9.50
Subgrade Modulus, K (psi/in) = 130
Modulus of Rupture, Sc (psi) = 650

Concrete Shoulder : No
Doweled Joints : Yes

Average Daily Truck Traffic, ADTT = 3680
Design Period, Years = 20
Load Safety Factor, LSF = 1.20

Axle Load (kips)      Axle Load (kips)
Single  Axle/1000      Tandem  Axle/1000
30.0    0.58            52.0    1.96
28.0    1.35            48.0    3.94
26.0    2.77            44.0    11.48
24.0    5.92            40.0    34.27
22.0    9.83            36.0    81.42
20.0    21.67           32.0    85.50
18.0    28.24           28.0    152.21
16.0    38.83           24.0    90.52
14.0    53.94           20.0    112.78
12.0    168.04          16.0    124.63
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圖二 PCAWIN 疲勞損壞分析結果輸出圖例一

PCA Thickness Design

File Edit Perform Analysis Compute Design Thickness Help

Fatigue Analysis Solutions

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*****
Axle      Expected  Equivalent  Stress  Allowable  Fatigue
Load, kips Repetitions Stress, psi Ratio Repetitions %
*****
Dual-Single
30.0      6310      393.8      0.606      26353      23.9
28.0      14688     369.0      0.568      75901      19.4
26.0      30137     344.2      0.530      232627     13.0
24.0      64409     319.3      0.491      1205097     5.3
22.0      106950    294.2      0.453      39972004    0.3
20.0      235768    269.0      0.414      Unlimited   0.0
18.0      307249    243.6      0.375      Unlimited   0.0
16.0      422467    218.1      0.336      Unlimited   0.0
14.0      586863    192.4      0.296      Unlimited   0.0
12.0      1836966   166.4      0.256      Unlimited   0.0

Dual-Tandem
52.0      21325     319.7      0.492      1164903     1.8
48.0      42867     296.5      0.456      23513514    0.2
44.0      124901    273.2      0.420      Unlimited   0.0
40.0      372855    249.8      0.384      Unlimited   0.0
36.0      885043    226.2      0.348      Unlimited   0.0
32.0      930233    202.5      0.312      Unlimited   0.0
28.0      1656033   178.6      0.275      Unlimited   0.0
24.0      984850    154.5      0.238      Unlimited   0.0
20.0      1227037   130.2      0.200      Unlimited   0.0
16.0      1355964   105.6      0.162      Unlimited   0.0

*****
Sum = 63.9

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圖三 PCAWIN 疲勞損壞分析結果輸出圖例二

PCA Thickness Design

File Edit Perform Analysis Compute Design Thickness Help

Erosion Analysis Solutions

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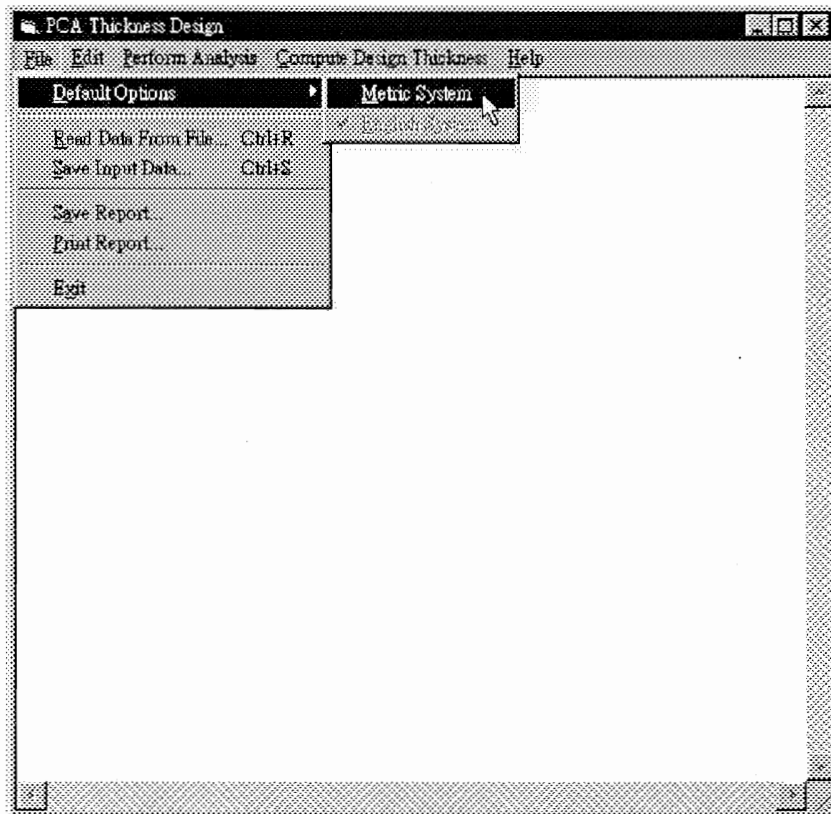
*****
Axle      Expected  Erosion    Power  Allowable  Erosion
Load, kips Repetitions Factor      Repetitions %
*****
Dual-Single
30.0      6310      4.59      37.927      1454607     0.4
28.0      14688     4.47      33.038      2206842     0.7
26.0      30137     4.34      28.487      3506784     0.9
24.0      64409     4.20      24.273      5929708     1.1
22.0      106950    4.05      20.396      10958174    1.0
20.0      235768    3.88      16.856      23291130    1.0
18.0      307249    3.70      13.654      64211588    0.5
16.0      422467    3.50      10.788      Unlimited   0.0
14.0      586863    3.26      8.260      Unlimited   0.0
12.0      1836966   3.00      6.068      Unlimited   0.0

Dual-Tandem
52.0      21325     4.72      44.292      921227      2.3
48.0      42867     4.58      37.740      1476168     2.9
44.0      124901    4.43      31.712      2503721     5.0
40.0      372855    4.27      26.208      4592541     8.1
36.0      885043    4.08      21.229      9467633     9.4
32.0      930233    3.88      16.773      23787906    3.9
28.0      1656033   3.65      12.842      91832816    1.8
24.0      984850    3.38      9.435      Unlimited   0.0
20.0      1227037   3.06      6.552      Unlimited   0.0
16.0      1355964   2.67      4.193      Unlimited   0.0

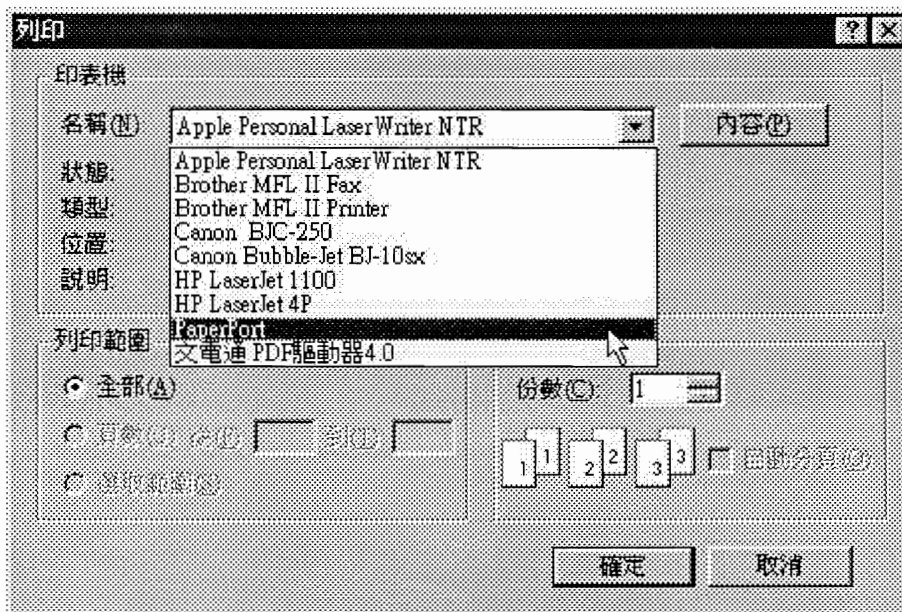
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Sum = 38.9

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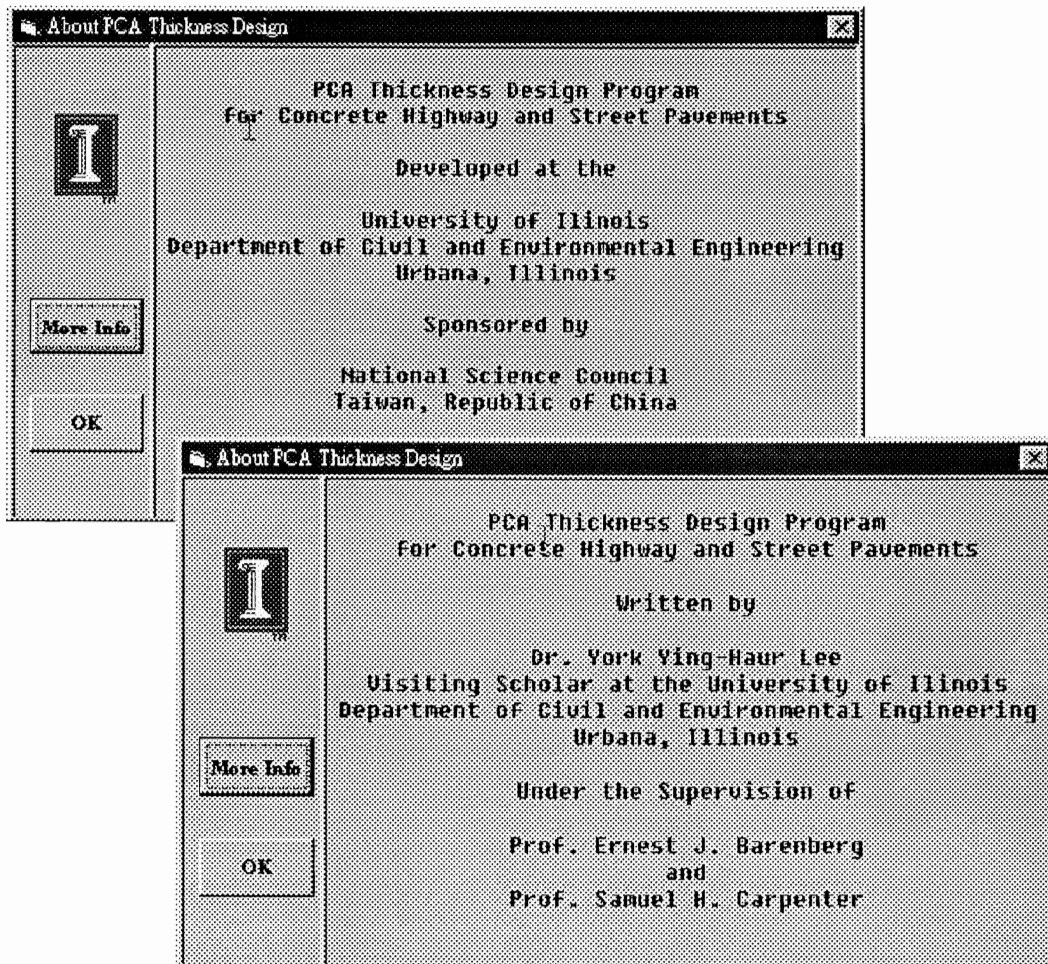
圖四 PCAWIN 侵蝕分析結果輸出圖例



圖五 PCAWIN 基本設定與檔案管理視窗圖例



圖六 PCAWIN 列印功能之印表機選擇視窗圖例



圖七 PCAWIN 程式說明視窗圖例